

Online Research @ Cardiff

This is an Open Access document downloaded from ORCA, Cardiff University's institutional repository: <http://orca.cf.ac.uk/110259/>

This is the author's version of a work that was submitted to / accepted for publication.

Citation for final published version:

Schanz, Tom, Tripathy, Snehasis and Sridharan, Asuri 2018. Volume change behaviour of swelling and non-swelling clays upon inundation with water and a low dielectric constant fluid. Applied Clay Science 158 , pp. 219-225. 10.1016/j.clay.2018.03.031 file

Publishers page: <https://doi.org/10.1016/j.clay.2018.03.031>
<<https://doi.org/10.1016/j.clay.2018.03.031>>

Please note:

Changes made as a result of publishing processes such as copy-editing, formatting and page numbers may not be reflected in this version. For the definitive version of this publication, please refer to the published source. You are advised to consult the publisher's version if you wish to cite this paper.

This version is being made available in accordance with publisher policies. See <http://orca.cf.ac.uk/policies.html> for usage policies. Copyright and moral rights for publications made available in ORCA are retained by the copyright holders.



Volume change behaviour of swelling and non-swelling clays upon inundation with water and a low dielectric constant fluid

Prof. Tom Schanz[†], Professor, tom.schanz@ruhr-uni-bochum.de

Dr. Snehasis Tripathy², Reader, TripathyS@cf.ac.uk

and

Prof. Asuri Sridharan³, Honorary Research Scientist, sridharanasuri@yahoo.com

Address:

† Deceased 12 October 2017. Formerly, Chair for Foundation Engineering, Soil- and Rock Mechanics, Ruhr-Universität Bochum, Germany

2 – Reader, Geoenvironmental Research Centre, School of Engineering, Cardiff University
Queens Buildings, West Groove, Newport Road, Cardiff CF24 3AA, UK

3 - Honorary Research Scientist, Indian National Science Academy, New Delhi, India,
Formerly, Professor, Indian Institute of Science Bangalore, India

Address of Corresponding author:

Dr. Snehasis Tripathy

School of Engineering, Cardiff University

Queens Buildings, West Groove, Newport Road, Cardiff CF24 3AA, UK

Tel: 0044-29208-79377

Fax: 0049-29208-74004

Email:TripathyS@cardiff.ac.uk

27 **1. Introduction**

28

29 Studies of the volume change behaviour of fine-grained soils upon exposure to
30 various types of fluid are of great interest, particularly while dealing with contaminated soils.
31 Contamination of soils in vadose zone may occur due to industrial activity, agricultural
32 chemicals, gasoline spillage, and improper disposal of waste. Similarly, mitigation strategies
33 adopted for soils contaminated with various organic contaminants demands understanding of
34 the soil-contaminant interaction at various stress states of soils.

35

36 The physico-chemical interaction between soil particles and their influence on the
37 volume change behaviour of fine grained soils depends upon the clay mineralogy and the
38 pore fluid characteristics (e.g., Sridharan and Rao, 1973; Lagaly 1989; Mitchell and Soga
39 2005). Organic fluids, chemicals, and petroleum products usually possess dielectric constant
40 values far lower than that of water. A review of the literature suggests that the influence of
41 organic fluids on the plasticity properties, compressibility, shear strength, and hydraulic
42 conductivity of saturated clays has been studied in detail by several researchers in the past
43 (Mesri and Olson 1971; Sridharan and Rao 1973; Sridharan and Rao 1979; Fernandez and
44 Quigley 1985; Lagaly 1989; Shang *et al.* 1994; Kaya and de Brito Galvao 1998; Kaya and
45 Fang 2005). However, studies concerning the volume change behaviour of compacted
46 unsaturated clays upon exposure to organic fluids, particularly in the context of the influence
47 of mineralogy, compaction conditions, and applied external stress are scarce in the literature.
48 In this study, laboratory one-dimensional swell or compression tests were carried out on two
49 clays with two types of fluid and at two different stress levels for studying the impacts of
50 polar and non-polar fluids, the magnitude of an applied stress, and initial compaction
51 conditions on the volume change behaviour of clays.

52

53 2. Background

54

55 During the wetting process and under a constant applied stress, the volume change of
56 unsaturated fine-grained soils is accompanied by a decrease in the shearing resistance at
57 inter-particle level brought about by a reduction in the soil suction, and by the changes in the
58 magnitude of the physico-chemical forces. As a result, a soil may exhibit swelling or
59 compression, or even the volume change may not occur under specific stress and hydraulic
60 boundary conditions. The physico-chemical forces are comprised of the electrical attractive
61 and the repulsive forces (Verwey and Overbeek 1948; Bolt 1956; van Olphen 1977;
62 Sridharan and Jayadeva 1982; Heuckel 1992; Mitchell and Soga 2005). The repulsive forces
63 that may get generated within clay-fluid systems are due to the interaction of electrical double
64 layers around the clay particles and hydration of ions and surfaces of clay particles (Madsen
65 and Müller-Vonmoos 1989; Laird 2006; Baille et al. 2010). These forces tend to cause an
66 increase in the volume of clays. The interaction between the edges of clay particles with the
67 negatively charged surfaces are electrostatic (Columbic) in nature. The primary and
68 secondary bonding (hydrogen bond and van der Waals bond) are the other possible sources of
69 attractive force that govern the shear resistance at inter-particle level. The short range inter-
70 particle force exists at very close spacing, whereas the long-range attractive forces (van der
71 Waals force) are considered to be sensitive to applied force and pore fluid properties.

72

73 The mobilised physico-chemical forces within clay systems to a great extent depend
74 upon the physical and chemical properties of the clays (i.e., specific surface area and type and
75 amount of exchangeable cations present in the clay), the bulk fluid properties (i.e., solvent
76 type, concentration, dielectric properties, and pH), the applied external stress, the soil suction,
77 the initial compaction conditions of the clay (i.e., dry density and water content), and the

78 temperature (Sridharan and Jayadeva 1982). The physico-chemical forces influence the fabric
79 in fine-grained soils, which in turn regulates the hydraulic conductivity, compressibility,
80 stiffness, and shear strength (Sridharan, 1991, Sridharan and Rao 1979; Fernandez and
81 Quigley 1985; Chen et al. 2000; Santamarina et al. 2001; Mitchell and Soga 2005; Wang and
82 Siu 2006). The fine-grained soil system is complex and the individual effects of various
83 parameters influencing the repulsive and attractive pressures cannot be readily separated.
84 However, it has been brought out in the literature that the dielectric constant of the pore
85 medium plays an important role in that, the attractive force varies inversely and the repulsive
86 force varies directly with the dielectric constant of the pore medium (Verwey and Overbeek
87 1948; van Olphen 1977; and Sridharan and Rao 1973 and 1979).

88

89 Adsorption of water molecules in the silicate layers of clay minerals tends to
90 eliminate the attractive pressure at the edge-face contacts in flocculated clay structure.
91 Development of the repulsive pressure during the wetting process causes further decrease in
92 the attractive pressure between the clay particles. A decrease in the attractive pressure gets
93 manifested on a reduction in the shearing resistance due to a reduction in effective contact
94 stress. Both the mobilized repulsive and attractive pressures are strongly dependent upon the
95 characteristics of the hydrating fluid. Partially saturated and fully saturated clay systems
96 possess attractive pressures lower than that occur in dry clays due to a partial or completion
97 of the surface and ion hydration processes (Sridharan 1968).

98

99 The exchangeable cations in clay are soluble in polar fluids (e.g., water and polar
100 organic liquids); however, are insoluble in non-polar organic liquids (e.g., heptane, toluene
101 etc.). The adsorption of polar organic liquid molecules in the interlayers occurs either as
102 replacement to water molecules in partially saturated clay systems or via the exchangeable

103 cations in fully dry clays (Grim 1968; Theng 1974; and van Olphen 1977). Sorption of non-
104 polar organic molecules in the interlayers of dry clays is usually insignificant, whereas the
105 process can be very slow or only limited adsorption may take place in the interlayers of
106 partially saturated clays (Grim 1968; Theng 1974; and Amarashinghe et al. 2009).

107

108 **3. Materials and methods**

109

110 Spergau kaolin and Calcigel benonite, both clays from Germany, were used in this
111 investigation. The properties of the clays are given in Table 1. The chosen clays possess a
112 wide range of physical and chemical properties. X-ray diffraction studies indicated that about
113 60% of the minerals in Calcigel bentonite were montmorillonite, whereas the remaining were
114 quartz, feldspar, dolomite, and calcite. Spergau kaolin was found to contain about 72%
115 kaolinite, 16% quartz, and 10% muscovite. Studies of the cation exchange complexes of the
116 clays indicated that more than 85% of the exchangeable cations in Calcigel bentonite were
117 Ca^{+2} and Mg^{+2} with small fractions of Na^{+} and other ions (Schanz and Tripathy 2009).
118 Spergau kaolin was found to contain Ca^{+2} , Mg^{+2} , and K^{+} .

119

120 One-dimensional swell and compression tests (ASTM 1986) were carried out in
121 conventional oedometers. All tests were carried out at a temperature of 20 ± 2 °C. Compacted
122 clay specimens were prepared at various water contents including from oven-dried clay
123 powder. Distilled water was used for preparing the clay-water mixtures. The clay-water
124 mixtures were stored in air-tight plastic bags for about a week in the laboratory for moisture
125 equilibration to take place. Compacted clay specimens were prepared corresponding to
126 predetermined dry densities with gentle tamping on the clay-water mixtures within the
127 oedometer rings. Extreme precautions were exercised in order to minimize the errors due to

128 side friction by lubricating the specimen rings with technical grade silicon grease. Filter
129 papers were used at the top and bottom of the specimens.

130

131 The details of the specimens tested for Spergau kaolin and Calcigel bentonite are
132 given in Tables 2 and 3, respectively. For both clays and for each combination of water
133 content-dry density, two different vertical pressures levels were considered, such as 25 and
134 100 kPa. For the clay specimens that were prepared from oven dried clay powder and at low
135 initial water contents (i.e., 0 and 12% for Spergau kaolin and 0 and 9.0% for Calcigel
136 bentonite), the targeted external loads were applied in a single step loading process, whereas
137 for the specimens that were prepared at higher water contents (61.5% for Spergau kaolin and
138 55.2 and 64.3% for Calcigel bentonite), a load increment ratio of one was adopted to attain
139 the required vertical pressure levels. The incremental loads were applied after an elapsed time
140 of more than three hours or upon completion of the primary consolidation phase, which ever
141 occurred earlier. Following the loading process, the specimens were inundated either with
142 water or heptane (C_7H_{16}). For every applied vertical stress, time-vertical deformation
143 readings were monitored using dial gauges of sensitivity of 0.001 mm and a total run of 25
144 mm. Similarly, the time-deformation readings (compression or swelling) were monitored
145 until the equilibrium conditions were attained. At each compaction conditions (see Tables 2
146 and 3), four specimens were tested. In total, thirty-six clay specimens were tested, sixteen for
147 Spergau kaolin and twenty for Calcigel bentonite.

148

149 The dielectric constant of water is 80.4, whereas that of heptane is 1.92 as against the
150 dielectric constant of air of 1.0. Additionally, water is a polar liquid, whereas heptane is a
151 non-polar organic liquid. Heptane is a highly flammable solvent. Extreme care was exercised
152 while handling the solvent by wearing gloves and a medical mask. The oedometer tests were

153 carried out in an isolated room with appropriate ventilation system. Similarly, appropriate
154 disposal methods were undertaken after completion of the laboratory tests.

155

156 Prior to testing for the clay specimens, the pressure-deformation characteristics of the
157 oedometers used were studied using a steel dummy following the procedure suggested by
158 ASTM (1998). Based on the pressure-deformation characteristics of the oedometers used, the
159 measured heights of the specimens at each loading steps were corrected for calculating the
160 actual vertical deformations. The vertical deformation at equilibrium under any given applied
161 vertical pressure is defined as the ratio of the change in height of a specimen upon either
162 swelling or compression to the height of the specimens prior to inundation and is expressed
163 as a percentage. The void ratio changes during swelling or compression of the specimens
164 were calculated based on height of solids method.

165

166 **4. Presentation of experimental results**

167

168 4.1 Volume change behaviour of Spergau kaolin

169

170 The elapsed time versus void ratio plots for Spergau kaolin specimens at four
171 different initial placement conditions, two different applied vertical pressures (i.e., 25 and
172 100 kPa), and with water and heptane as the inundating fluids are shown in Figs. 1 and 2. The
173 vertical deformations of the specimens at equilibrium are shown in Table 2.

174

175 Following inundation of the specimens either with water or heptane, except two
176 specimens that exhibited a small volume increase upon inundated with heptane (water
177 contents of 0 and 23.3%, dry density 1.11 Mg/m^3), all other specimens exhibited compression

178 (Table 2, Figs. 1 and 2). The compression deformation of the specimens occurred within
179 about 15 to 25 minutes following the inundation process in all cases.

180

181 The test results of the initially unsaturated specimens showed three distinct trends in
182 terms of the volume change (Table 2, Figs. 1a, 1b and 2a), such as (i) at any applied vertical
183 pressure (25 or 100 kPa), the compression deformation was significantly smaller for the
184 specimens that were inundated with heptane than that occurred with water, (ii) the vertical
185 deformation decreased with an increase in the initial compaction dry density and with an
186 increase in the water content for both inundating fluid types, and (iii) the vertical deformation
187 increased with an increase in the applied stress for both fluid types.

188

189 For the saturated clay specimens with an initial water content of 61.5% (Fig. 2b), the
190 vertical deformations at any given applied pressure and with both water and heptane as the
191 inundating fluids were very nearly similar clearly indicating that the pore-fluid type has
192 limited impact on the volume change behaviour of saturated kaolinites.

193

194 4.2 Volume change behaviour of Calcigel bentonite

195

196 The elapsed time versus void ratio plots for the specimens of Calcigel bentonite at
197 five different initial placement conditions, two different applied vertical pressures (i.e., 25
198 and 100 kPa), and with water and heptane as the inundating fluids are shown in Figs. 3, 4,
199 and 5. The initial compaction conditions of the specimens, the inundation fluids used, and the
200 applied vertical pressures are shown in Figs. 3 to 5. The vertical deformations of the
201 specimens at equilibrium are given in Table 3.

202

203 Following inundation either with water or heptane, except four specimens with high
204 initial void ratios of 1.94 and 1.8 that underwent slight compression at an applied stress of
205 100 kPa, all other specimens (Table 3) exhibited swelling. The test results of the initially
206 unsaturated specimens showed three distinct trends in terms of the volume change (Figs. 3 to
207 5 and Table 3), such as (i) the swelling deformation was significantly greater for the
208 specimens that were inundated with water as against their counterparts that were inundated
209 with heptane, (ii) with an increase in the applied stress during inundation with any fluid, the
210 deformation behaviour of the specimens was accompanied by either a decrease in the
211 swelling deformation or an increase in the compression deformation, or even specimens that
212 exhibited swelling at 25 kPa, underwent compression at 100 kPa, (iii) for the specimens with
213 an initial water content of 0%, an increase in the compaction dry density caused an increase
214 in swelling deformation, whereas swelling deformation decreased with an increase in the
215 initial water content for the same compaction dry density (see results for dry density of 1.10
216 Mg/m^3 in Table 3).

217

218 The test results presented in Figs. 3, 4, and 5 clearly indicated that unlike the
219 deformation behaviour of the unsaturated specimens of Spergau kaolin, in which case the
220 compression deformation invariably increased with an increase in applied vertical pressure
221 and for both inundation fluids used (Figs. 1 and 2, Table 2), the deformation behaviour was
222 found to be reversed in the case of Calcigel bentonite. In the latter case, the swelling
223 deformation decreased with an increase in the vertical pressure. Additionally, the influence of
224 fluid type and applied vertical pressure was found to be quite significant on the volume
225 change behaviour of the specimens that were initially saturated (Fig. 5 and Table 3). This was
226 not evident in the case of saturated Spergau kaolin specimens (Figs. 2*b* and Table 2).

227

228 **5. Influence of physico-chemical forces on the volume change of clays**

229

230 Table 4 summarises the influence of a decrease in the dielectric constant of the
231 inundating fluid, an increase in the applied stress during the inundation process, an increase
232 in the compaction water content, and an increase in the compaction dry density on the vertical
233 deformation of clay specimens observed in this study. The possible reasons for the volume
234 change of clays are presented in Table 4.

235 Further, the results can also be explained by the changes in the effective stress that has
236 been brought out by changes in normal stress, repulsive and attractive forces. It has been now
237 widely accepted that Terzaghi's concept of effective stress provides a satisfactory basis for
238 understanding the strength and deformation characteristics of saturated soils, which can be
239 stated as:

240

$$241 \quad \sigma' = \sigma - u \quad (1)$$

242

243 where σ' = effective stress, σ = applied external stress or the total stress and u = pore water
244 pressure.

245

246 It may be noted that σ' is the contact stress at mineral to mineral contact zone, which
247 is also called inter-granular stress. While one can discuss at greater length the nature of this
248 contact, for purpose of brevity, it can be said that the role of contact is to transfer the stress. It
249 has been brought out earlier that both electrical attractive and repulsive forces exist between
250 clay particles. Since the fine grained soils are normally composed of clays, the existence of
251 attractive and repulsive forces in the soil-water system is inevitable. The studies of Sridharan

252 (1968) and Sridharan and Rao (1973, 1979) resulted in the proposition of modified effective
 253 stress concept as given in eq. (2).

254

$$255 \quad \bar{c} = \bar{\bar{\sigma}} a_m = \sigma - u - R + A \quad (2)$$

256

257 For a saturated system, where \bar{c} is the average contact stress, $\bar{\bar{\sigma}}$ is actual contact stress at
 258 mineral - to - mineral level, a_m is area fraction over which $\bar{\bar{\sigma}}$ acts or percentage area through
 259 which $\bar{\bar{\sigma}}$ acts (non-dimensional), σ = the external applied stress, u = pore water pressure, R =
 260 average repulsive pressure acting throughout the area, A = average attractive pressure acting
 261 throughout the area.

262

263 Equation (2) can be written as

264

$$265 \quad \bar{c} = \bar{\bar{\sigma}} a_m = \sigma' + \sigma'' \quad (3)$$

266

267 where σ'' is the intrinsic effective stress and σ' is the conventional effective stress.

268

269 It may further be stated that the effect of σ and A is to bring the particles closer to each
 270 other. The effect of positive pore water pressure and R is to keep the particles away from each
 271 other. If the pore water pressure is negative (i.e., capillary pressure operative in partly
 272 saturated soils), its role is to bring the particles closer to each other. The average contact
 273 stress or the intergranular stress (\bar{c}) between particles, is defined as the modified effective
 274 stress and it is hypothesised that \bar{c} is the stress controlling the shearing resistance and
 275 volume changes that take place in soil-water system. In fine-grained soils / clayey soils, the
 276 attractive and repulsive forces cannot be neglected, especially when the water content and the

277 soil plasticity are high. Since the clay-water system is complex, quantitative determination of
278 R and A becomes difficult for real systems. However, qualitative evaluation could be done.
279 The validity of eqns. (2) and (3) have been qualitatively studied extensively considering the
280 volume change behaviour (Sridharan and Rao 1973; Sridharan et al. 1973; Sridharan 2003:),
281 the strength behaviour (Sridharan et al. 1971; Sridharan and Rao 1979; Allam and Sridharan
282 1981; Sridharan et al. 1983; Sridharan and Prakash, 1999b), the shrinkage phenomena
283 (Sridharan and Rao 1971), the secondary compression behaviour (Sridharan and Rao 1982)
284 and the sediment formation (Sridharan and Prakash 1999a).

285

286 The thickness of the electrical double layer that is formed around the clay particles
287 are influenced by several factors (Mitchell and Soga 2005), such as the characteristics of the
288 hydrating fluid, the type and amount of exchangeable cation present in the clay, the specific
289 surface area, the applied external stress, and the temperature (Verwey and Overbeek 1948;
290 van Olphen 1977; Sridharan and Rao 1979; and Sridharan and Jayadeva 1982). The thickness
291 of the electrical double layer controls the swelling, compressibility, shear strength, and
292 hydraulic conductivity in clays. The factors that cause an increase in the thickness of
293 electrical double layer are also responsible for a higher magnitude of swelling deformation in
294 expansive clays resulting in a decrease in the modified effective stress. Similarly, the factors
295 that cause a decrease in the thickness of electrical double layer aid in an increasing the
296 modified effective stress and increasing the shearing resistance and cause a decrease in the
297 compressibility and the hydraulic conductivity.

298

299 The attractive force varies inversely and the repulsive force varies directly with the
300 dielectric constant of the pore fluid (Sridharan and Rao 1973 and 1979). Therefore, a
301 decrease in the dielectric constant of the pore fluid caused two opposite effects for both clays

302 in this study when the inundating fluid was heptane, such as an increase in the attractive
303 pressure and a decrease in the repulsive pressure and thus an increase in the modified
304 effective stress. A greater mobilised attractive pressure with heptane as the inundating fluid
305 caused an increase in the modified effective stress and two distinct effects, such as that (i) the
306 interparticle shearing resistance did not decrease as that occurred with water as the inundating
307 fluid thereby causing a lesser compression of the specimens of Spergau kaolin and (ii) since
308 the electrical double layer thickness was reduced in case of Calcigel bentonite, the clay
309 exhibited a lesser swelling as compared to that occurred with water as the inundating fluid.
310 The magnitude of applied stress on the clays can be considered to have two effects, such as
311 an increase in the applied stress acts against the interparticle shearing resistance to cause a
312 greater deformation in case of Spergau kaolin and it acts as a restraint against the expansion
313 of the electrical double layer thereby causing a reduction in the swelling deformation in case
314 of Calcigel bentonite.

315

316 An increase in the initial compaction water content and its influence on the
317 deformation behaviour of the clays can be considered separately for the two inundating fluids
318 considered in this study (i.e., water and heptane). An increase in the compaction water
319 content causes a reduction in the inter-particle shearing resistance since the attractive pressure
320 is inversely proportional to the dielectric constant of the pore fluid. Additionally, an increase
321 in the compaction water content causes the hydration of the surfaces and the ions. A further
322 increase in the compaction water content above that is required for surface and ion hydration
323 aids in the formation of the electrical double layer. The increased volume of the clay due to
324 the expansion of the electrical double layer depends upon the volume of water available and
325 the type of clay mineral present. With the inundating fluid as water, the changes in the
326 vertical deformation in case Spergau kaolin was found to be minor since the pore fluid

327 remained unchanged. The swelling deformation in the case of Calcigel bentonite also reduced
328 since a part of the swelling process was already completed during the specimen preparation
329 stage. The clay specimens of Calcigel bentonite exhibited further swelling primarily due to
330 the available of water which caused a further expansion of the electrical double layer.

331

332 Replacement of water by heptane can be considered when the inundating fluid used
333 was heptane. This would tend to cause an increase in the inter-particle shearing resistance
334 primarily due to an increase in the attractive pressure and a decrease in thickness of the
335 electrical double layer. A replacement of molecules of water by heptane for kaolinite is
336 expected to cause no significant change in terms of the deformation, but due to a change in
337 the magnitude of the attractive pressure the clay may tend to flocculate causing an increase in
338 the volume (see test results for specimens at water content = 12% in Table 2). A decrease in
339 the thickness of electrical double layer in case of Calcigel bentonite reduced the swelling
340 deformation of the clay. Saturated specimens exhibited slight swelling deformation which
341 indicated that replacement of water with heptane will increase the electrical attractive forces
342 resulting in a relatively flocculent fabric which tend to get eliminated as the applied stress
343 increased (see test results of initially saturated specimens in Table 3).

344

345 An increase in the compaction dry density is associated with an increase in the applied
346 stress that overcomes the inter-particle shearing resistance and tends to produce a more
347 oriented fabric perpendicular to the direction of the applied load. A decrease in the void ratio
348 brings the clay particles closer towards each other which in turn, causes an increase in the
349 attractive pressure. Ignoring the orientation of clay particles and the difference in the
350 magnitude of the attractive pressure, under any given applied stress, clay specimens with
351 different initial compaction dry densities but with same initial water content may exhibit

352 different vertical deformations, but should attain the same void ratio upon saturation. A
353 decrease in the compression deformation in case of Spergau kaolin is attributed due to an
354 increase in the attractive pressure, whereas an increase in the swelling deformation due to an
355 increase in the compaction dry density in the case of Calcigel bentonite is attributed due to
356 expansion of the electrical double layer in response to the applied stress during the inundation
357 process.

358

359 Thus it can be seen that the volume change (compression or swelling) behaviour of
360 both the clays in this study can be explained by the changes in the intrinsic effective stress,
361 defined by equations (2) and (3), brought out by the changes in the electrical attractive and
362 repulsive pressures (influenced by the fluid types by their di-electric constant values), the
363 negative pore pressures and the externally applied stress.

364

365 **6. Conclusions**

366

367 The one-dimensional volume change behaviour of kaolinite and montmorillonite-rich
368 clays is presented in this paper. Several initially unsaturated and saturated clay specimens
369 were inundated either with water or heptane at applied vertical pressures of 25 and 100 kPa.

370

371 The test results clearly showed that the physico-chemical forces within the clay-fluid
372 systems are significantly influenced by the mineralogy of clays, the properties of the
373 hydrating and the inundating fluids, the compaction dry density and water content, and the
374 applied stress during the inundation process. The kaolinite-rich clay in this study exhibited
375 low compression with both water and heptane, whereas the montmorillonite clay exhibited
376 significant swelling (up to 23%) with water, whereas in the kaolinite clay exhibited swelling

377 up to about 0.9%. The changes in the volume of the clays were found to be much smaller
378 with heptane (attributed to strong electrical attractive forces) as compared that occurred with
379 water. The volume change behaviour of the clays are explained in the light of the modified
380 effective stress and the interplay of attractive and repulsive pressures.

381

382 **Acknowledgments:** The Visiting International Professor grant received from Research
383 School PLUS, Ruhr University Bochum, Germany is gratefully acknowledged. The second
384 author acknowledges the financial support provided by the Welsh Government and Higher
385 Education Funding Council for Wales through the Se'r Cymru National Research Network
386 for Low Carbon, Energy and Environment.

387

388 **References**

389

390 Allam, M.M, Sridharan, A. 1981. Effect of wetting and drying on shear strength. Journal of
391 the Geotechnical Engineering Division, ASCE, Vol. 107, No.GT4, 421- 438.

392 Amarasinghe, P.M., Katti, K.S., Katti, D.R. 2009. Nature of organic fluid–montmorillonite
393 interactions: An FTIR spectroscopic study. Journal of Colloid and Interface Science
394 337, 97-105.

395 American Society for Testing and Materials (ASTM) 1986. Standard test method for one-
396 dimensional swell or settlement potential of cohesive soils, ASTM D4546, In 1986
397 Annual Book of ASTM Standards, Vol. 04.08, Soil and Rock; Building Stones,
398 Section 4, ASTM, Philadelphia, PA.

399 American Society for Testing and Materials (ASTM) 1998. Standard Test Method for One-
400 Dimensional Consolidation Properties of Soils, ASTM D2435-96, In 1998 Annual

- 401 Book of ASTM Standards, Vol. 04.08, American Society for Testing and Materials,
402 West Conshohocken, PA.
- 403
- 404 Baille, W., Tripathy, S., Schanz, T. 2010. Swelling pressures and one-dimensional
405 compressibility behaviour of bentonite at large pressure. *Applied Clay Science* 48 (3),
406 324-333.
- 407 Bolt, G.H. 1956. Physico-chemical analysis of the compressibility of pure clays.
408 *Géotechnique* 6 (2), 86–93.
- 409 Chen, J., Anandarajah, A., Inyang, H. 2000. Pore fluid properties and compressibility of
410 kaolinite. *Journal of Geotechnical and Geoenvironmental Engineering, ASCE*, 126
411 (9), 798–807.
- 412 Fernandez, F., Quigley, R.M. 1985. Hydraulic conductivity of natural clays permeated with
413 simple liquid hydrocarbons. *Canadian Geotechnical Journal* 22, 205-214.
- 414 Grim, R.E. 1968. *Clay Mineralogy*, McGraw-Hill, New York.
- 415 Heuckel, T.A. 1992. Water-mineral interaction in hygromechanics of clays exposed to
416 environmental load: a mixture-theory approach. *Canadian Geotechnical Journal* 29,
417 1071-1086.
- 418 Kaya, A., de Brito Galvao, C. 1998. Hydraulic conductivity of fine-grained soils permeated
419 with water-organic liquid mixtures. *Contaminated and dielectric land. The Proc. of*
420 *GREEN 2: the second international symposium on Geotechnics Related to the*
421 *Environment held in Krakow, Poland, September 1997, Ed. R. W. Sarsby, Thomas*
422 *Telford, UK.*
- 423 Kaya, A., Fang, H-Y. 2005. Experimental evidence of reduction in attractive and repulsive
424 forces between clay particles permeated with organic liquids. *Canadian Geotechnical*
425 *Journal* 42 (2), 632-640.

- 426 Lagaly, G. 1989. Principles of flow of kaolin and bentonite dispersions. *Applied Clay*
427 *Science* 4 (2), 105-123.
- 428
- 429 Laird, D.A. 2006. Influence of layer charge on swelling of smectites. *Applied Clay Science*
430 34 (1-4), 74-87.
- 431 Madsen, F.T., Müller-Vonmoos M. 1989. The swelling behaviour of clays. *Applied Clay*
432 *Science* 4 (2), 143-156.
- 433 Mesri, G., Olson, R.E. 1971. Consolidation characteristics of montmorillonite. *Geotechnique*
434 21 (4), 341-352.
- 435 Mitchell, J.K., Soga, K. 2005. *Fundamentals of soil behaviour*. 3rd ed. Wiley, New York.
- 436 Santamarina, J.C., Klein, K. A., Fam, M. A. 2001. *Soils and waves*. John Wiley & Sons Ltd.,
437 New York.
- 438 Schanz, T., Tripathy S. 2009. Swelling pressure of a divalent-rich bentonite: Diffuse double-
439 layer theory revisited, *Water Resources Research* 45, W00C12
440 10.1029/2007WR006495.
- 441 Shang, J.Q., Lo, K.Y., Quigley, R.M. 1994. Quantitative determination of potential
442 distribution in Stern–Gouy double-layer model. *Canadian Geotechnical Journal* 31,
443 624–636.
- 444 Sridharan, A. 1968. Some studies on the strength of partly saturated clays. PhD thesis, Purdue
445 University, Indiana.
- 446 Sridharan, A. 1991. Engineering behaviour of clays - A fundamental approach. *Indian*
447 *Geotechnical Journal* 21, 1-136.
- 448 Sridharan, A. 2003. Volume change behaviour of fine grained soils. *Unsaturated soils:*
449 *Experimental studies*. Proc. International Conference from experimental evidence

- 450 towards numerical modeling of unsaturated soils Sept. 18 – 19, 2003, Bauhaus-
451 university Weimar, Germany, Springer Proceedings in Physics, Vol. 1, 209-226.
- 452 Sridharan, A., Allam, M.M. 1982. Volume change behaviour of desiccated soils. Journal of
453 the Geotechnical Engineering Division, ASCE, Vol. 108, No.GT8, 1057-1071.
- 454 Sridharan, A., Jayadeva, M.S. 1982. Double layer theory and compressibility of clays.
455 *Géotechnique* 32 (2), 133–144.
- 456 Sridharan, A., Jayadeva, M.S. 1982. Double layer theory and compressibility of clays.
457 *Géotechnique* 32 (2), 133–144.
- 458 Sridharan, A., Prakash, K (1999a) Influence of clay mineralogy and pore medium chemistry
459 on clay sediment formation. *Canadian Geotechnical Journal* 36 (5), 961-966.
- 460 Sridharan, A., Prakash, K. (1999b) Mechanisms controlling the undrained shear strength
461 behaviour of clays. *Canadian Geotechnical Journal* 36 (6), 1030 -1038.
- 462 Sridharan, A., Rao, G.V. 1973. Mechanisms controlling volume change of saturated clays
463 and the role of the effective stress concept. *Geotechnique* 23 (2), 359-382.
- 464 Sridharan, A., Rao, G.V. 1979. Shear strength behaviour of saturated clays and the role of the
465 effective stress concept. *Geotechnique* 29 (2), 177-193.
- 466 Sridharan, A., Rao, G.V. 1979. Shear strength behaviour of saturated clays and the role of the
467 effective stress concept. *Geotechnique* 29 (2), 177-193.
- 468 Sridharan, A., Rao, G.V., and Pandian, R.S. 1973. Volume change behaviour of partly
469 saturated clays during soaking and the role of effective stress concept. *Soils and*
470 *Foundations* 13 (3), 1-15.
- 471 Theng, B.K., 1974. *The Chemistry of Clay-Organic Reactions* Scientist. Soil Bureau,
472 Department of Scientific and Industrial Research, Lower Hutt, New Zealand, Halsted
473 Press.

- 474 van Olphen, H. 1977. An introduction to clay colloid chemistry: for clay technologists,
475 geologists and soil scientists. Interscience, New York.
- 476 Verwey, E.J.W., Overbeek, J.Th.G. 1948. Theory of the stability of lyophobic colloids.
477 Elsevier, Amsterdam.
- 478 Wang, Y.-H., Siu, W.-K. 2006. Structure characteristics and mechanical properties of
479 kaolinite soils. I. Surface charges and structural characterizations. Canadian
480 Geotechnical Journal 43, 587-600.
- 481

482 **Table 1** Properties of the clays used in the study

Properties	Spergau kaolin	Calcigel bentonite
Specific gravity of soil solids, G_s	2.62	2.80
Liquid limit, w_L (%)	53.4	178.0
Plastic limit, w_P (%)	30.1	56.0
Shrinkage limit, w_S (%)	22.3	8.8
Specific surface area, S (m^2/g)	14.0	650.0
Base exchange capacity, B (meq/100g)	8.0	74.0

483

484

485 **Table 2** Details of Speregau kaolin specimens tested

486

Initial compaction conditions				Inundation	Vertical deformation	
Water	Dry	Void	Degree	fluid	(%)	at
content (%)	density	ratio	of		vertical	applied
	(Mg/m ³)		saturation		stress	
			(%)		25 kPa	100
						kPa
0	1.02	1.58	0	Water	-13.42	-17.63
0	1.02	1.58	0	Heptane	-0.23	-2.99
0	1.11	1.35	0	Water	-8.14	-11.87
0	1.11	1.35	0	Heptane	0.43	-0.13
12.0	1.11	1.35	23.3	Water	-7.57	-11.50
12.0	1.11	1.35	23.3	Heptane	0.34	-0.60
61.5	1.00	1.61	100	Water	-0.43	-0.47
61.5	1.00	1.61	100	Heptane	-0.44	-0.41

487

488

489 **Table 3** Details of Calcigel bentonite specimens tested
 490

Initial compaction conditions				Inundation	Vertical deformation	
Water	Dry	Void	Degree	fluid	(%)	at
content (%)	density	ratio	of		vertical	applied
	(Mg/m ³)		saturation		stress	
					25 kPa	100
						kPa
0	0.95	1.94	0	Water	16.01	2.7
0	0.95	1.94	0	Heptane	-0.49	-0.55
0	1.10	1.54	0	Water	22.80	7.21
0	1.10	1.54	0	Heptane	0.91	0.37
9.0	1.10	1.54	16.4	Water	19.87	6.92
9.0	1.10	1.54	16.4	Heptane	0.83	0.28
55.2	1.10	1.54	100	Water	7.6	0.86
55.2	1.10	1.54	100	Heptane	0.16	0.00
64.3	1.00	1.80	100	Water	3.2	-0.70
64.3	1.00	1.80	100	Heptane	0.29	-0.22

491
 492

493 **Table 4** Influence of various factors on the vertical deformation of unsaturated clays upon
 494 saturated under applied stress

Reference	Vertical deformation		Remarks
	Kaolinite	Montmorillonite	
Decrease in dielectric constant of inundating fluid	Compression decreases	Swelling decreases	Attractive pressure increases/Double layer thickness decreases
Increase in applied stress during inundation	Compression increases	Swelling decreases	Interparticle shearing resistance exceeded/Double layer expansion restricted
Increase in compaction water content	Compression decreases	Swelling decreases	Attractive pressure decreases/Surface and ion hydration processes and development of double layer
Increase in compaction dry density	Compression decreases	Swelling increases	Attractive pressure increases/Repulsive pressure increases

495
 496

497 **Figure captions**

498 **Fig. 1** Water content versus total suction plot for the clays used in this study

499 **Fig. 2** Time-deformation behaviour of Spergau kaolin specimens

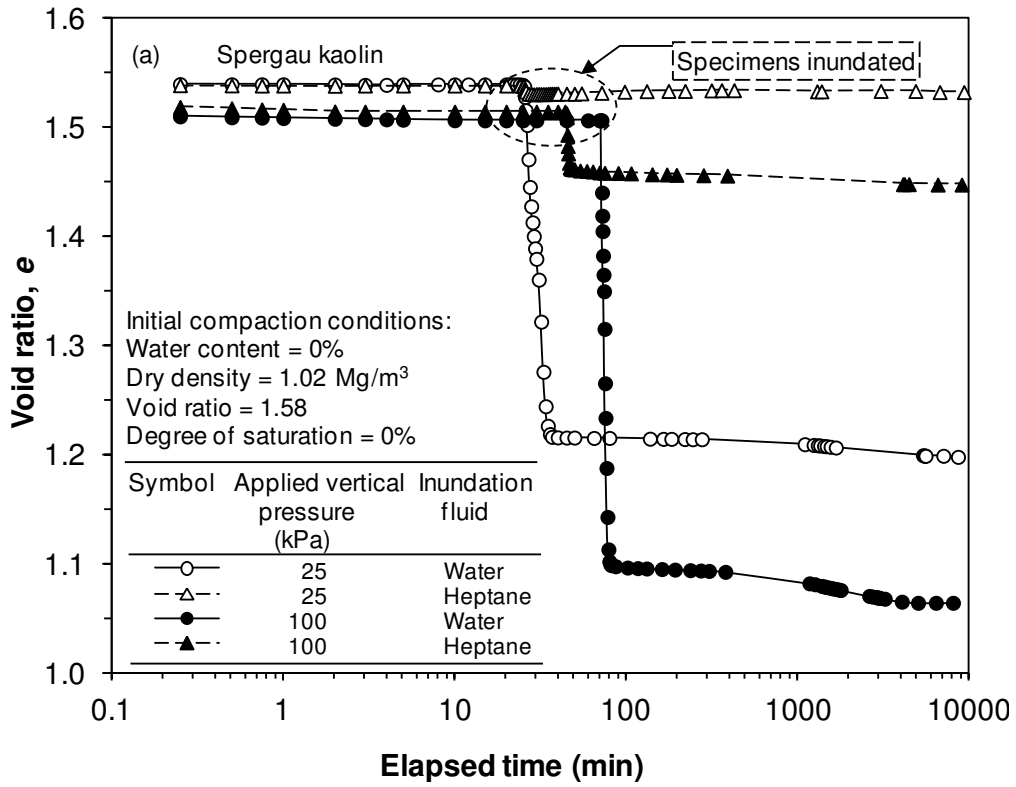
500 **Fig. 3** Summary of vertical deformations of Spergau kaolin specimens

501 **Fig. 4** Time-deformation behaviour of Calcigel bentonite specimens

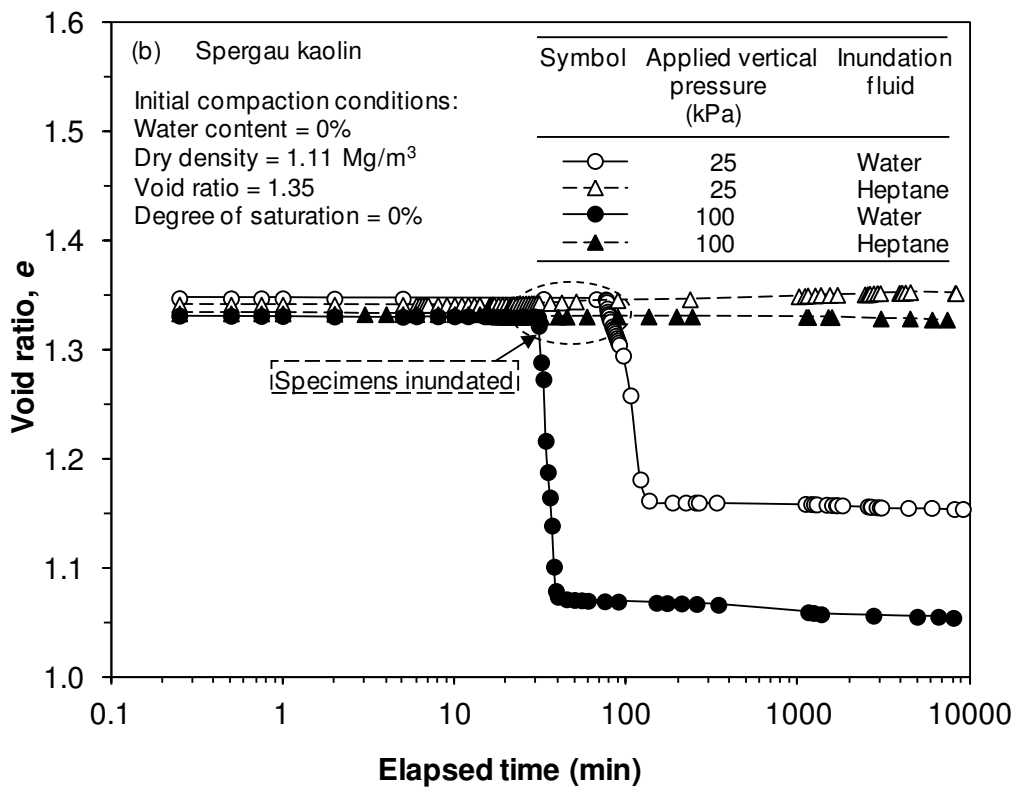
502 **Fig. 5** Summary of vertical deformations of Calcigel bentonite specimens

503

504
505



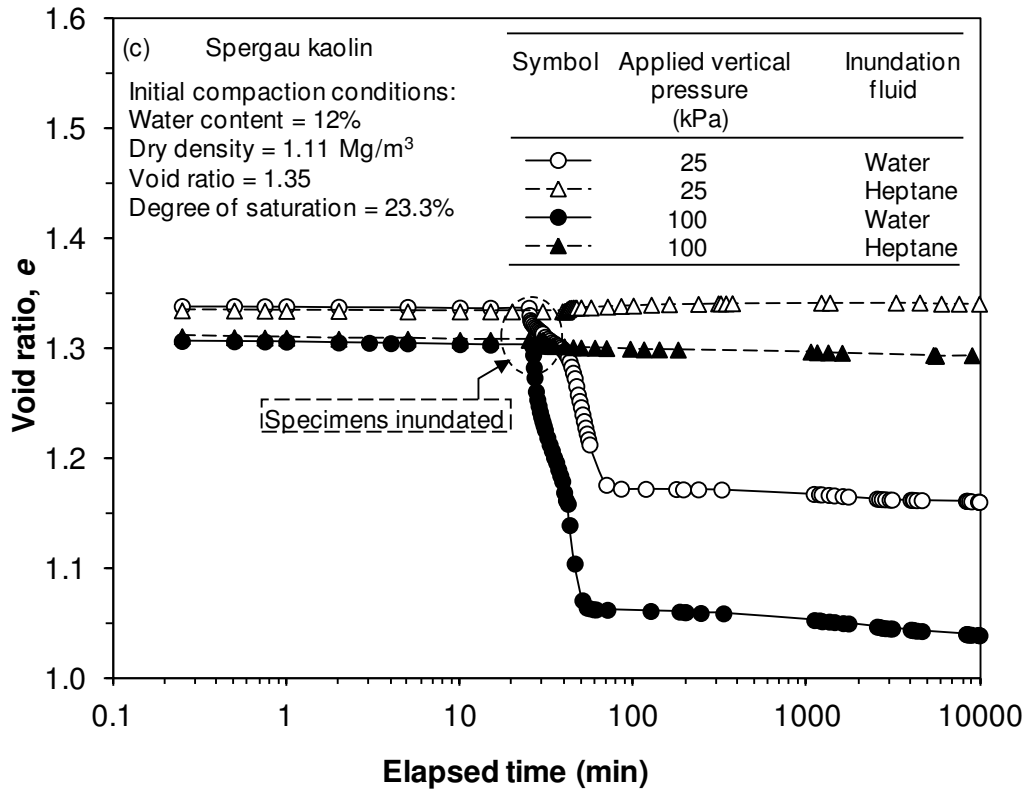
506



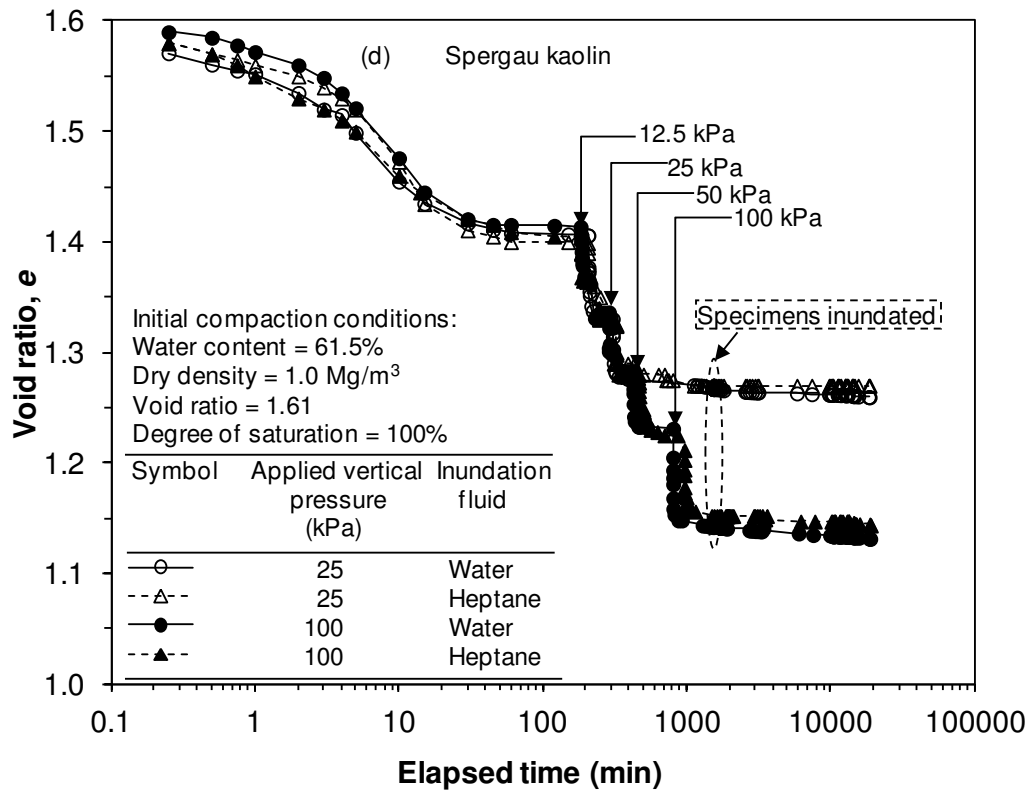
507

508
509

Fig. 1 Time-deformation behaviour of Spergau kaolin specimens (initial water content = 0%)



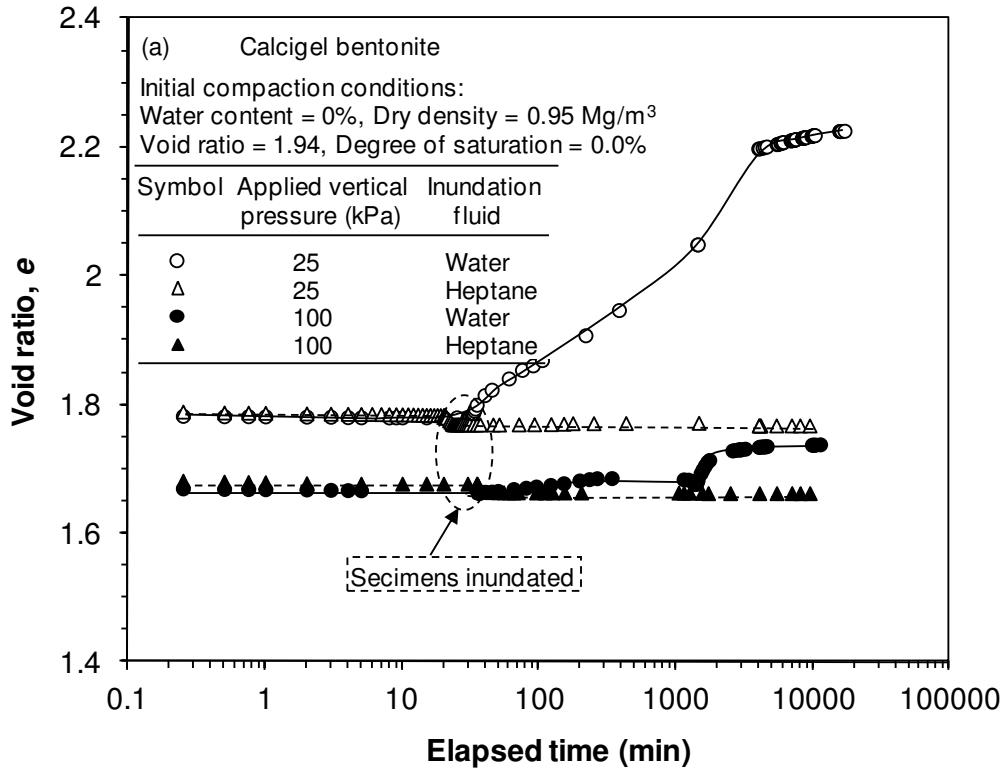
510



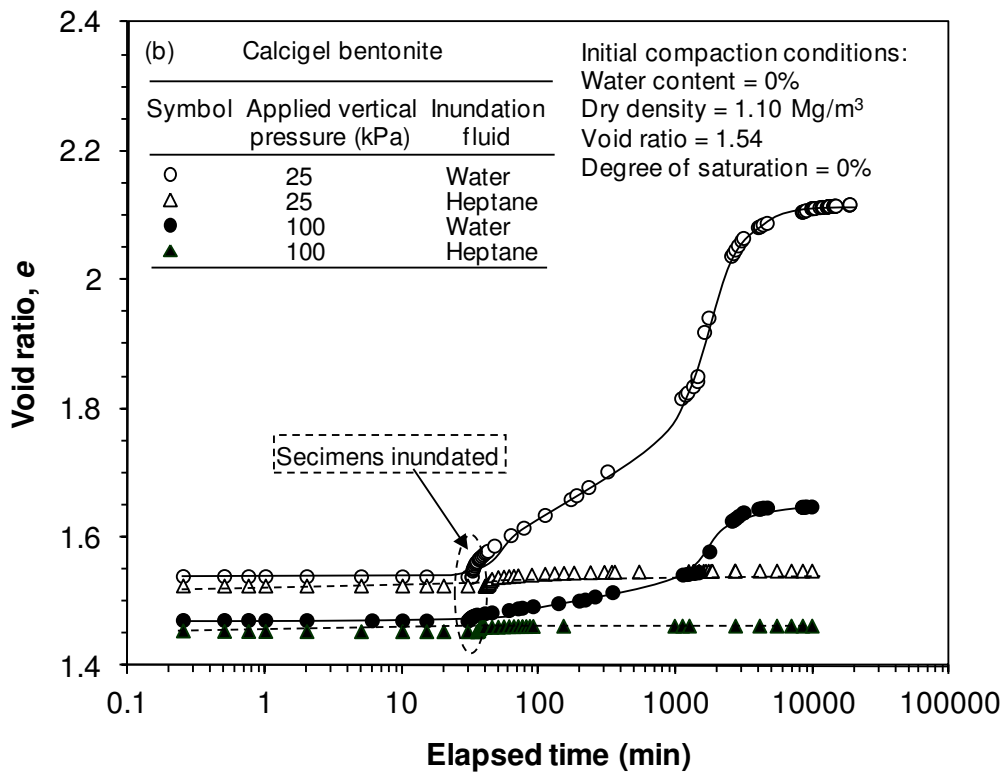
511

512 **Fig. 2** Time-deformation behaviour of Spergau kaolin specimens with initial water contents
 513 of (a) 12% and (b) 61.5%

514

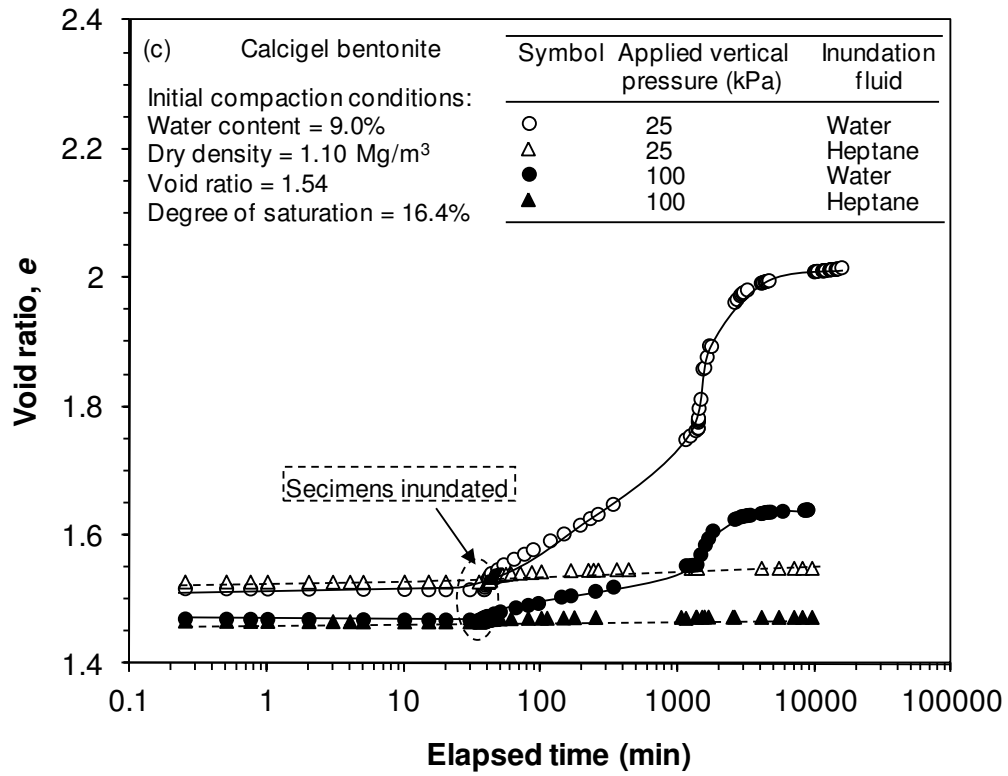


515



516

517 **Fig. 3** Time-deformation behaviour of Calcigel bentonite specimens (initial water content =
518 0%)

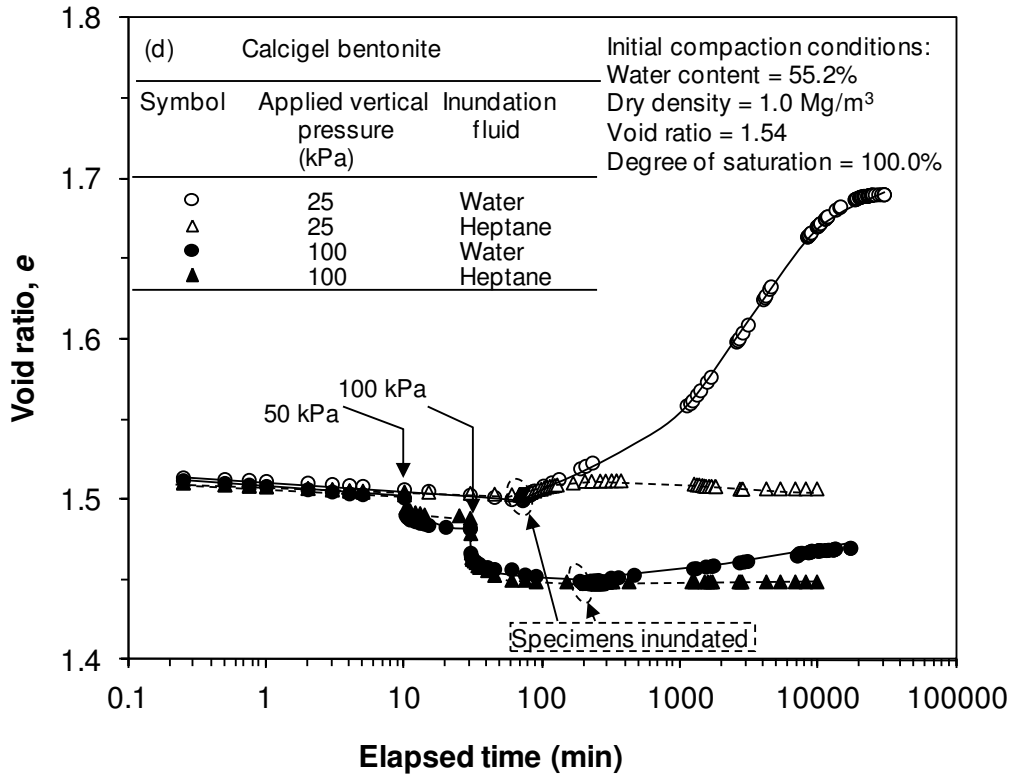


519

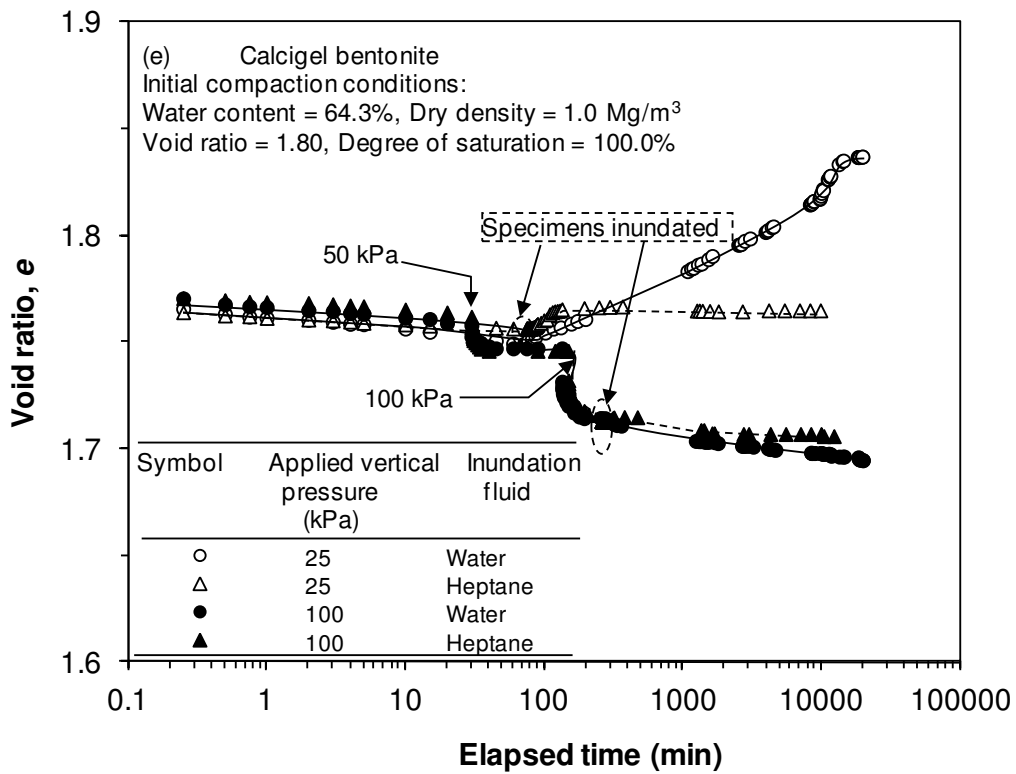
520 **Fig. 4** Time-deformation behaviour of Spergau kaolin specimens (initial water content = 9%)

521

522



523



524

525 **Fig. 5** Time-deformation behaviour of Calcigel bentonite specimens with initial water
 526 contents of (a) 55.2% and (b) 64.3%

Volume change behaviour of swelling and non-swelling clays upon inundation with water and a low dielectric constant fluid

Prof. Tom Schanz[†], Professor, tom.schanz@ruhr-uni-bochum.de

Dr. Snehasis Tripathy², Reader, TripathyS@cf.ac.uk

and

Prof. Asuri Sridharan³, Honorary Research Scientist, sridharanasuri@yahoo.com

Address:

[†] Deceased 12 October 2017. Formerly, Chair for Foundation Engineering, Soil- and Rock Mechanics, Ruhr-Universität Bochum, Germany

² – Reader, Geoenvironmental Research Centre, School of Engineering, Cardiff University Queens Buildings, West Groove, Newport Road, Cardiff CF24 3AA, UK

³ - Honorary Research Scientist, Indian National Science Academy, New Delhi, India, Formerly, Professor, Indian Institute of Science Bangalore, India

Highlights

- Volume change behaviour of clays with water and heptane as the inundating fluids is presented.
- Mineralogy, dielectric properties, compaction conditions, and stress impact the volume change.
- Mobilised attractive and repulsive forces dictate compression and swelling in unsaturated clays.

**Volume change behaviour of swelling and non-swelling clays upon inundation with
water and a low dielectric constant fluid**

Tom Schanz [†], Snehasis Tripathy, and Asuri Sridharan

Abstract: Studies of the volume change behaviour of saturated and unsaturated fine-grained soils upon exposure to various types of fluid are of significant interest while dealing with contaminated soils. In this study, the one-dimensional volume change behaviour of two clays (Spergau kaolin and Calcigel bentonite) with dominant minerals as kaolinite and montmorillonite was studied by inundating several initially unsaturated and saturated clay specimens with water and a non-polar organic liquid (heptane). For both cases, applied vertical pressures of 25 and 100 kPa were considered under oedometric conditions. The magnitude of compression and swelling deformations of the clays were found to be strongly dependent upon the mineralogy of clays, dielectric properties of the pore-fluid, initial compaction conditions, and applied stress during the wetting process. The test results indicated distinct interactions between unsaturated clays and water. Specimens of Spergau kaolin exhibited compression with water, whereas specimens of Calcigel bentonite exhibited swelling. Compression and swelling deformations of the clays with heptane as the inundating liquid remained within about $\pm 1.0\%$ for both clays indicating a very minor interaction between unsaturated clays and molecules of this non-polar organic liquid. The test results emphasized the significance of attractive and repulsive forces and their impact on the volume change behaviour of clays of different mineralogy.

Keywords: minerology of clays; swelling; compression; pore-fluid; waste containment; laboratory tests

[†] Deceased 12 October 2017